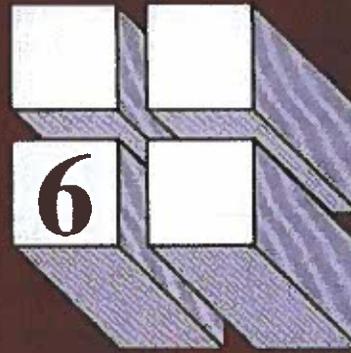
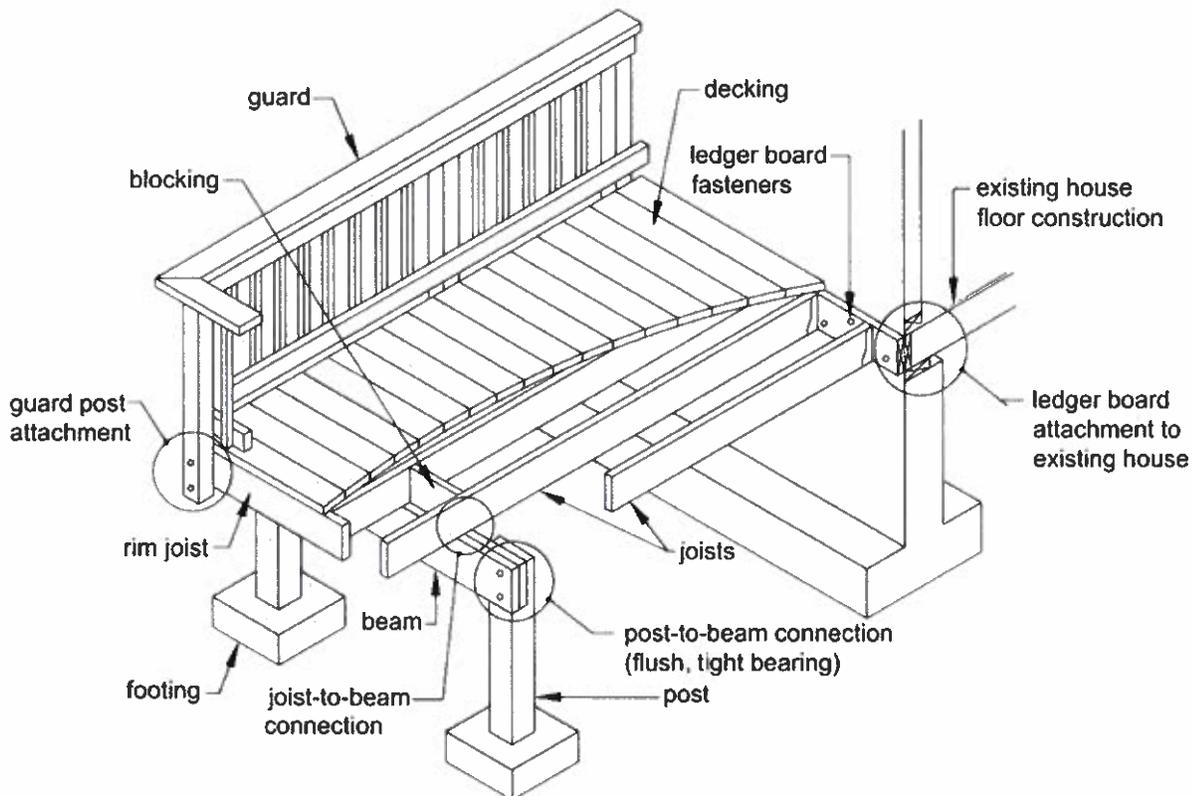


Design for Code Acceptance



Prescriptive Residential Wood Deck Construction Guide

Based on the 2015 International Residential Code



Where applicable, provisions and details contained in this document are based on the *International Residential Code (IRC)* [bracketed text references applicable sections of the *IRC*]. Prescriptive construction methods recommended meet or exceed minimum requirements of the *IRC*. Provisions that are not found in the *IRC* are recommended as good industry practice. Where differences exist between provisions of this document and the *IRC*, provisions of the *IRC* shall apply. This document is not intended to preclude the use of other construction methods or materials. All construction and materials must be *approved* by the authority having jurisdiction. Every effort has been made to reflect the language and intent of the *IRC*. However, no assurance can be given that designs and construction made in accordance with this document meet the requirements of any particular jurisdiction.

CONTENTS

MINIMUM REQUIREMENTS & LIMITATIONS	2	LEDGER BOARD FASTENERS	15
DECKING REQUIREMENTS	3	NON-LEDGER DECKS - VERTICAL LOADS	17
JOIST SIZE	3	DECK LATERAL LOADS	17
BEAM SIZE & ASSEMBLY REQUIREMENTS	5	GUARD REQUIREMENTS	19
DECK FRAMING PLAN	8	GUARD POST ATTACHMENTS	19
JOIST-TO-BEAM CONNECTION	9	STAIR REQUIREMENTS	20
JOIST HANGERS	9	STAIR HANDRAIL REQUIREMENTS	22
POST REQUIREMENTS	10	STAIR FOOTING REQUIREMENTS	22
RIM JOIST REQUIREMENTS	11	STAIR LIGHTING REQUIREMENTS	22
FOOTINGS	11	FRAMING AT CHIMNEY OR BAY WINDOW	23
LEDGER ATTACHMENT REQUIREMENTS	13		
PROHIBITED LEDGER ATTACHMENTS	15		

MINIMUM REQUIREMENTS & LIMITATIONS

1. This document applies to single level residential wood decks that are attached to the house to resist lateral forces. [R507.2.4]
2. Overall deck length shall be equal to or less than overall deck width. See DECK FRAMING PLAN for definition of deck length and width.
3. Minimum post size is 6x6 nominal and maximum post height shall be in accordance with Table 4.
4. All lumber shall be identified by the grade mark of, or certificate of inspection issued by, an *approved* lumber grading or inspection bureau or agency (www.alsc.org). All lumber and glued laminated timber shall be a naturally durable species (such as Redwood or Western Cedars where 90 percent or more of the width of each side is heartwood); or be preservatively treated with an *approved* process in accordance with American Wood Protection Association standards (Table 1) [R317 and R318]. All lumber in contact with the ground shall be *approved* preservative treated wood suitable for ground contact. [R317.1.2] All cuts shall be field treated with an *approved* preservative (such as copper naphthenate) [R402.1.2].
5. All nails shall meet the requirements of *ASTM F 1667*. Threaded nails as stated in this document include helical (spiral) and annular (ring-shank) nails. Wood screws shall meet the requirements of *ANSI/ASME B18.6.1*. Bolts and lag screws shall meet the requirements of *ANSI/ASME B18.2.1*.
6. Throughout this document, 1/2" diameter bolts and lag screws are specified for various connections. Edge distance and spacing requirements are based on 1/2" diameter fasteners. If larger (or smaller) fasteners are specified, edge distance and spacing shall be adjusted.
7. To resist corrosion, the following is required [R317.3]:
 - All screws, bolts, washers, nuts, and nails for use with preservative treated wood shall be hot-dipped zinc-coated galvanized steel, stainless steel, silicon bronze, or copper. Hot-dipped galvanized fasteners shall meet the requirements of *ASTM A 153*, Class D for fasteners 3/8" diameter and smaller or Class C for fasteners with diameters over 3/8". Stainless steel driven fasteners shall be in accordance with the material requirements of *ASTM F 1667*.
 - Fasteners other than nails and timber rivets shall be permitted to be of mechanically deposited zinc-coated steel with coating weights in accordance with *ASTM B 695*, Class 55, minimum.
 - All connectors (joist hangers, cast-in-place post anchors, etc.) shall be galvanized or shall be stainless steel. Hardware to be hot-dipped prior to fabrication shall meet *ASTM A 653*, G-185 coating. Hardware to be hot-dipped galvanized after fabrication shall meet *ASTM A 123*.
 - Fasteners and connectors exposed to salt water or located within 300 feet of a salt water shoreline shall be stainless steel grade 304 or 316.
 - Fasteners and connectors shall be of the same corrosion-resistant material.
 - Other coated or non-ferrous fasteners or hardware shall be *approved* by the authority having jurisdiction.
8. Decks supporting large concentrated loads such as hot tubs are beyond the scope of this document.
9. This document does not apply to decks which will experience snow loads, snow drift loads, or sliding snow loads that exceed 40 psf.
10. Lateral load resistance is limited to the prescriptive provisions of R507.2.4 of the IRC. Alternative loads and detailing shall be *approved* by the authority having jurisdiction.

11. Flashing shall be corrosion-resistant metal [R703.8] of minimum nominal 0.019-inch thickness or *approved* non-metallic material. Aluminum should not be used in direct contact with lumber treated with preservatives that contain copper such as ACQ, Copper Azole, or ACZA.

12. Decks shall not be used or occupied until final inspection and approval is obtained.

13. This document is not intended to preclude the use of other construction methods or materials not described herein.

Table 1. Common Species and Use Categories for Decay Resistance.¹

	Species	Above Ground	Ground Contact
Preservative-Treated ²	Southern Pine	X	X
	Douglas Fir-Larch	X	X
	Hem-Fir	X	X
	SPF	X	
	Ponderosa Pine	X	X
	Red Pine	X	X
	Redwood	X	X
	Western Cedars	X	
Naturally Durable ³	Redwood	X	
	Western Cedars	X	

1. Use categories listed in Table 1 are based on the American Wood Protection Association (AWPA) *Book of Standards*.

2. Above Ground – UC3B; Ground Contact – UC4A.

3. Naturally durable species with 90% heartwood in width on each side need not be treated per minimum requirements.

DECKING REQUIREMENTS

All decking material shall be composed of dimension lumber (2" nominal thickness) or span rated decking in accordance with the American Lumber Standard Committee *Policy for Evaluation of Recommended Spans for Span Rated Decking Products (November 5, 2004)*. Attach decking to each joist with 2-8d threaded nails or 2-#8 screws. Space decking boards approximately 1/8" apart. See Figure 11 for decking connection requirements at the rim joist. Decking placement may range from an angle perpendicular to the joists to an angle of 45 degrees to the joists. Each segment of decking must bear on a minimum of 3 joists (or 3 supports).

Decking not meeting these requirements may be substituted when the product has been *approved* by the

authority having jurisdiction; however, connections equivalent to those shown for lumber or span rated decking are assumed.

JOIST SIZE

The span of a joist, L, is measured from the face of support at one end of the joist to the face of support at the other end of the joist and does not include the length of the overhangs, L_o. Use Table 2 to determine allowable joist span, L_j, based on lumber size and joist spacing. Joist span, L, must be less than or equal to allowable joist span, L_j. Overhang length is the lesser of allowable overhang, L_o, or one fourth the joist span, L/4. See Figure 1 and Figure 2 for joist span types.

Table 2. Maximum Joist Spans and Overhangs.¹

Species	Size	Joist Spacing (o.c.)					
		12"	16"	24"	12"	16"	24"
		Allowable Span ² (L _J)			Allowable Overhang ³ (L _O)		
Southern Pine	2x6 ⁶	9' - 11"	9' - 0"	7' - 7"	1' - 0"	1' - 1"	1' - 3"
	2x8	13' - 1"	11' - 10"	9' - 8"	1' - 10"	2' - 0"	2' - 4"
	2x10	16' - 2"	14' - 0"	11' - 5"	3' - 1"	3' - 5"	2' - 10"
	2x12	18' - 0" ⁷	16' - 6"	13' - 6"	4' - 6"	4' - 2"	3' - 4"
Douglas Fir-Larch, Hem-Fir, Spruce-Pine-Fir ⁴	2x6 ⁶	9' - 6"	8' - 4"	6' - 10"	0' - 11"	1' - 0"	1' - 2"
	2x8	12' - 6"	11' - 1"	9' - 1"	1' - 8"	1' - 10"	2' - 2"
	2x10	15' - 8"	13' - 7"	11' - 1"	2' - 10"	3' - 2"	2' - 9"
	2x12	18' - 0" ⁷	15' - 9"	12' - 10"	4' - 4"	3' - 11"	3' - 3"
Redwood, Western Cedars, Ponderosa Pine ⁵ , Red Pine ⁵	2x6 ⁶	8' - 10"	8' - 0"	6' - 10"	0' - 9"	0' - 10"	0' - 11"
	2x8	11' - 8"	10' - 7"	8' - 8"	1' - 5"	1' - 7"	1' - 9"
	2x10	14' - 11"	13' - 0"	10' - 7"	2' - 5"	2' - 7"	2' - 8"
	2x12	17' - 5"	15' - 1"	12' - 4"	3' - 7"	3' - 9"	3' - 1"

1. Assumes 40 psf live load, 10 psf dead load, No. 2 grade, and wet service conditions.
2. Assumes L/360 deflection.
3. Maximum allowable overhang cannot exceed L/4 or ¼ of actual main span. Assumes cantilever length/180 deflection with 220 lb point load (See Figure 1A and Figure 2).
4. Incising assumed for Douglas Fir-Larch, Hem-Fir, and Spruce-Pine-Fir.
5. Design values based on northern species with no incising assumed.
6. Ledger shall be a minimum of 2x8 nominal. Joists and rim joists to which guard posts are attached shall be a minimum of 2x8 nominal.
7. Joist length prescriptively limited to 18'-0" for footing design.

Figure 1A. Joist Span – Joists Attached at House and Bearing Over Beam.

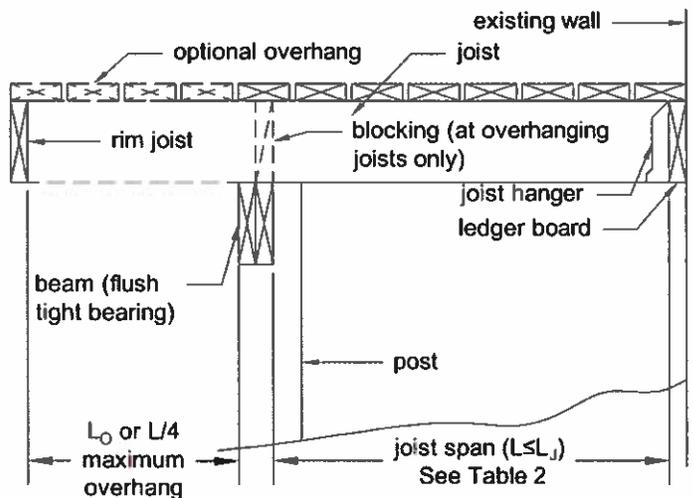


Figure 1B. Joist Span – Joists Attached at House and to Side of Beam.

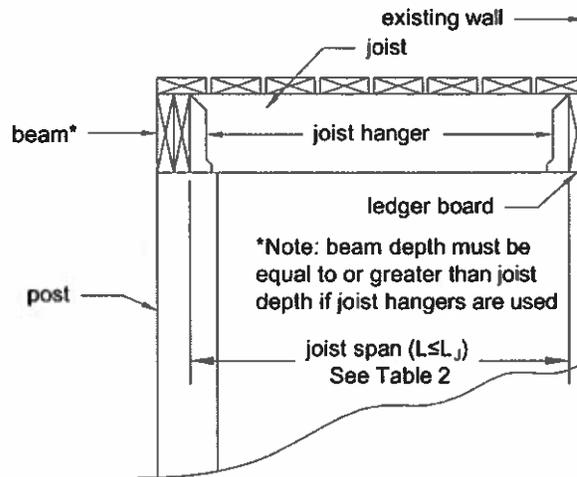
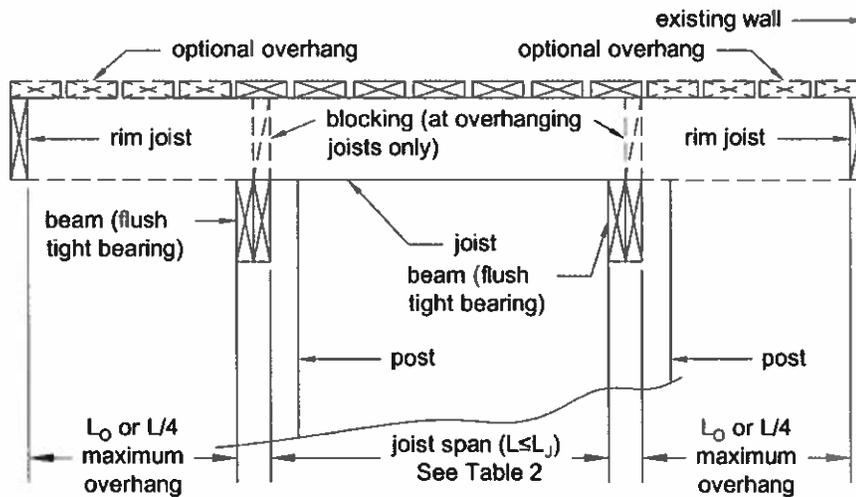


Figure 2. Joist Span – Non-Ledger Deck.



BEAM SIZE & ASSEMBLY REQUIREMENTS

Deck beam spans shall be in accordance with Table 3 and can extend past the post face up to $L_B/4$ as shown in Figure 3. Joists may bear on the beam and extend past the beam face up to the lesser of L_0 or $L/4$ as shown in Figures 1A and 2, or the joists may attach to the side of the beam with joist hangers as shown in Figure 1B.

Joists shall not frame in from opposite sides of the same beam. See JOIST-TO-BEAM CONNECTION details, Figure 6.

Where multiple 2x members are used, the deck's beam is assembled by attaching the members identified in Table 3A in accordance with Figure 4 [Table R602.3(1)].

Table 3A. Dimension Lumber Deck Beam Spans (L_B)¹ Supporting a Single Span of Joists with or without Overhangs.

Species	Size ⁴	Joist Spans (L) Less Than or Equal to:						
		6'	8'	10'	12'	14'	16'	18'
Southern Pine	2-2x6	6' - 8"	5' - 8"	5' - 1"	4' - 7"	4' - 3"	4' - 0"	3' - 9"
	2-2x8	8' - 6"	7' - 4"	6' - 6"	5' - 11"	5' - 6"	5' - 1"	4' - 9"
	2-2x10	10' - 1"	8' - 9"	7' - 9"	7' - 1"	6' - 6"	6' - 1"	5' - 9"
	2-2x12	11' - 11"	10' - 4"	9' - 2"	8' - 4"	7' - 9"	7' - 3"	6' - 9"
	3-2x6	7' - 11"	7' - 2"	6' - 5"	5' - 10"	5' - 5"	5' - 0"	4' - 9"
	3-2x8	10' - 7"	9' - 3"	8' - 3"	7' - 6"	6' - 11"	6' - 5"	6' - 1"
	3-2x10	12' - 9"	11' - 0"	9' - 9"	8' - 9"	8' - 3"	7' - 8"	7' - 3"
	3-2x12	15' - 0"	13' - 0"	11' - 7"	10' - 6"	9' - 9"	9' - 1"	8' - 7"
Douglas Fir-Larch ² , Hem-Fir ² , Spruce-Pine-Fir ² , Redwood, Western Cedars, Ponderosa Pine ³ , Red Pine ³	3x6 or 2-2x6	5' - 2"	4' - 5"	3' - 11"	3' - 7"	3' - 3"	2' - 10"	2' - 6"
	3x8 or 2-2x8	6' - 7"	5' - 8"	5' - 1"	4' - 7"	4' - 3"	3' - 10"	3' - 5"
	3x10 or 2-2x10	8' - 1"	7' - 0"	6' - 3"	5' - 8"	5' - 3"	4' - 10"	4' - 5"
	3x12 or 2-2x12	9' - 5"	8' - 2"	7' - 3"	6' - 7"	6' - 1"	5' - 8"	5' - 4"
	4x6	6' - 2"	5' - 3"	4' - 8"	4' - 3"	3' - 11"	3' - 8"	3' - 5"
	4x8	8' - 2"	7' - 0"	6' - 3"	5' - 8"	5' - 3"	4' - 11"	4' - 7"
	4x10	9' - 8"	8' - 4"	7' - 5"	6' - 9"	6' - 3"	5' - 10"	5' - 5"
	4x12	11' - 2"	9' - 8"	8' - 7"	7' - 10"	7' - 3"	6' - 9"	6' - 4"
	3-2x6	7' - 1"	6' - 5"	5' - 9"	5' - 3"	4' - 10"	4' - 6"	4' - 3"
	3-2x8	9' - 5"	8' - 3"	7' - 4"	6' - 8"	6' - 2"	5' - 9"	5' - 5"
	3-2x10	11' - 9"	10' - 2"	9' - 1"	8' - 3"	7' - 7"	7' - 1"	6' - 8"
	3-2x12	13' - 8"	11' - 10"	10' - 6"	9' - 7"	8' - 10"	8' - 3"	7' - 10"

1. Assumes 40 psf live load, 10 psf dead load, L/360 simple span beam deflection limit, cantilever length/180 deflection limit, No. 2 grade, and wet service conditions.
2. Incising assumed for Douglas Fir-Larch, Hem-Fir, and Spruce-Pine-Fir.
3. Design values based on northern species with no incising assumed.
4. Beam depth must be equal to or greater than joist depth if joist hangers are used (see Figure 6, Option 3).

Table 3B. Glued Laminated Timber Beam Spans (L_B)¹ Supporting a Single Span of Joists with or without Overhangs.

Stress Class ²	Width ³	Depth ⁴	Joist Spans (L) Less Than or Equal to:						
			6'	8'	10'	12'	14'	16'	18'
Balanced or Unbalanced 20F-1.5E And Higher Grade Cedar	3-1/2"	9-1/2"	12'-2"	10'-6"	9'-4"	8'-6"	7'-10"	7'-4"	6'-11"
		11-7/8"	15'-2"	13'-1"	11'-8"	10'-8"	9'-10"	9'-2"	8'-8"
		14"	17'-10"	15'-5"	13'-9"	12'-7"	11'-7"	10'-10"	10'-2"
	5-1/4"	11-7/8"	18'-0" ⁵	16'-8"	14'-10"	13'-7"	12'-6"	11'-8"	11'-0"
		14"	18'-0" ⁵	18'-0" ⁵	17'-6"	15'-11"	14'-9"	13'-9"	13'-0"
		16"	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	16'-10"	15'-9"	14'-10"
Unbalanced 24F-1.8E Douglas Fir-Larch or Southern Pine	3-1/2"	9-1/2"	13'-11"	12'-1"	10'-9"	9'-10"	9'-1"	8'-6"	8'-0"
		11-7/8"	17'-5"	15'-1"	13'-5"	12'-3"	11'-4"	10'-7"	10'-0"
		14"	18'-0" ⁵	17'-9"	15'-10"	14'-5"	13'-4"	12'-6"	11'-9"
	5-1/4"	11-7/8"	18'-0" ⁵	18'-0" ⁵	17'-1"	15'-7"	14'-5"	13'-6"	12'-8"
		14"	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	17'-0"	15'-10"	14'-11"
		16"	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	17'-0"
Balanced 24F-1.8E Douglas Fir-Larch or Southern Pine	3-1/2"	9-1/2"	13'-11"	12'-7"	11'-8"	11'-0"	10'-5"	9'-11"	9'-7"
		11-7/8"	17'-5"	15'-10"	14'-8"	13'-9"	13'-1"	12'-6"	12'-0"
		14"	18'-0" ⁵	18'-0" ⁵	17'-4"	16'-3"	15'-5"	14'-9"	14'-2"
	5-1/4"	11-7/8"	18'-0" ⁵	18'-0" ⁵	17'-4"	16'-3"	15'-5"	14'-9"	14'-2"
		14"	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	17'-6"	16'-9"
		16"	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵	18'-0" ⁵

1. Assumes 40 psf live load, 10 psf dead load, L/360 simple span beam deflection limit, cantilever length/180 deflection limit. Glued laminated timber shall be of naturally durable species or treated with an oil-borne or water-borne preservative in accordance with AWPA U1. See MINIMUM REQUIREMENTS & LIMITATIONS. Spans for glued laminated timber beams not treated with oil-borne preservatives shall be multiplied by 0.89.
2. Preservative treated structural composite lumber of equal or greater capacity can be substituted.
3. Beam widths of 3-1/8" or wider can be used for the tabulated 3-1/2" width, and beam widths of 5-1/8" or wider can be used for the tabulated 5-1/4" width.
4. Beam depth must be equal to or greater than joist depth if joist hangers are used (see Figure 6, Option 3).
5. Beam span prescriptively limited to 18'-0" for footing design.

Figure 3. Beam Span.

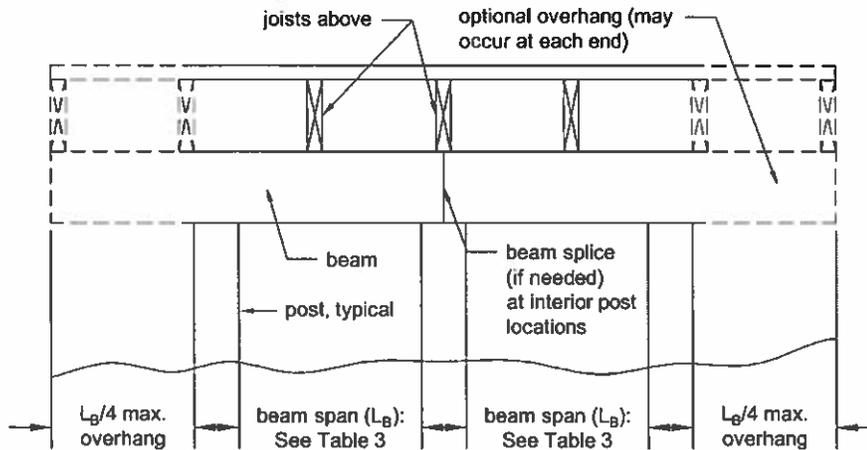
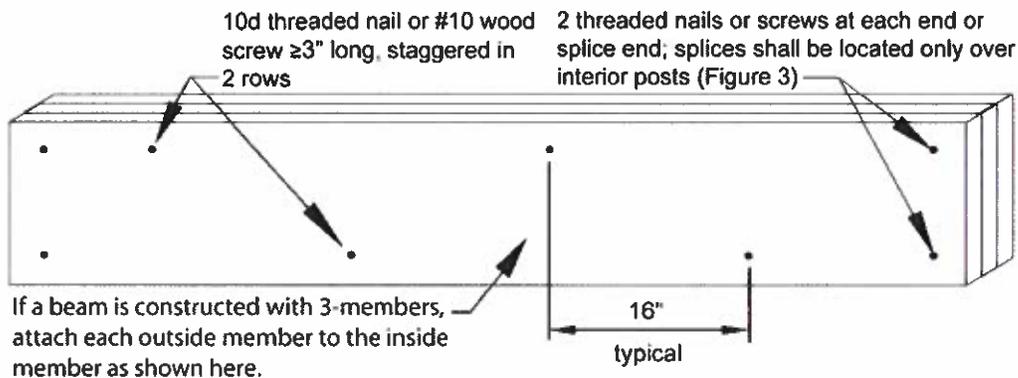


Figure 4. Beam Assembly Details.

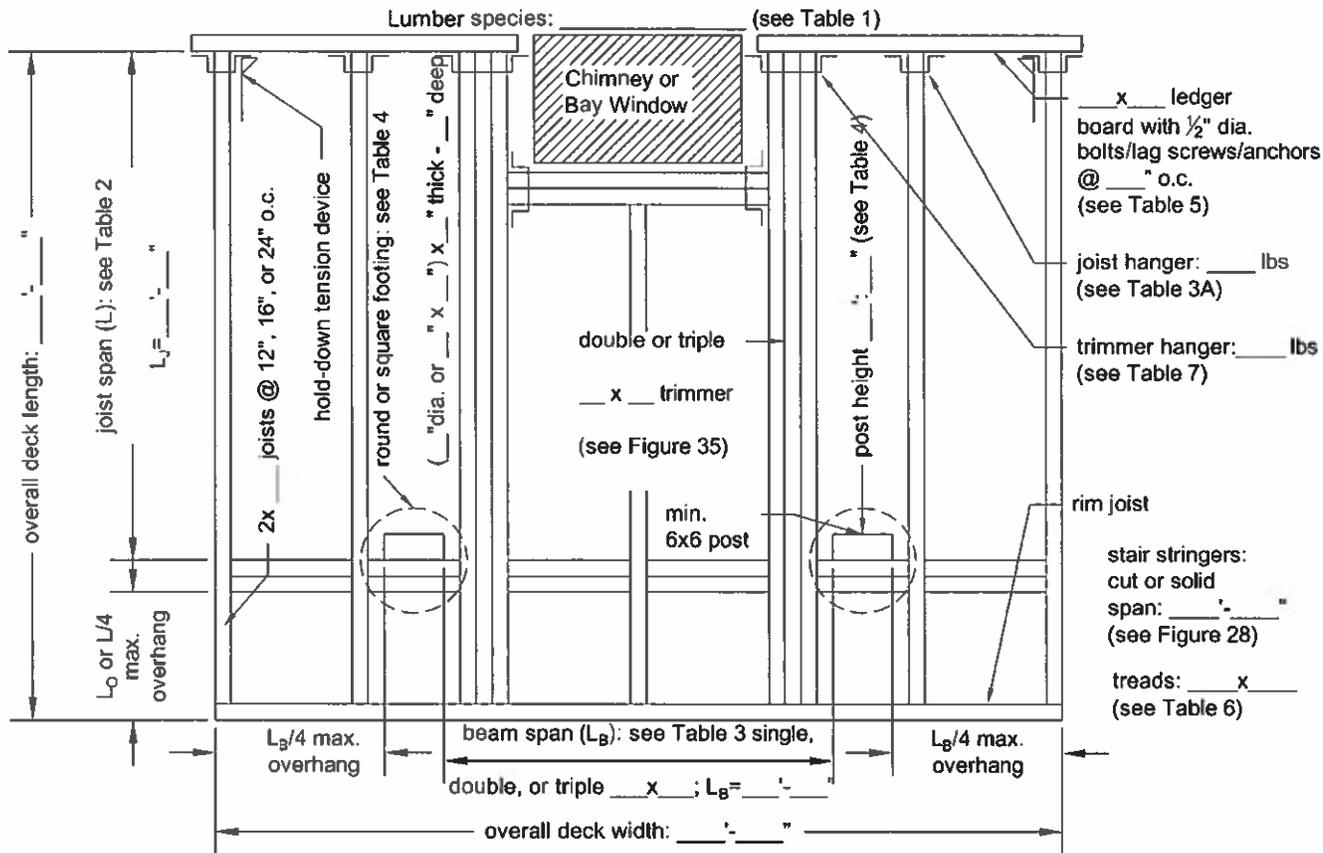


DECK FRAMING PLAN

A framing plan shows the width, length, joist and beam layout; the location of the ledger board, posts, and footings; and the type, size, and spacing of the ledger board fasteners.

The overall deck length shall be equal to or less than the overall deck width. Stairs and stair landings shall not be included in determining the overall deck length or width. See Figure 5 for an example of a typical deck framing plan.

Figure 5. Typical Deck Framing Plan.

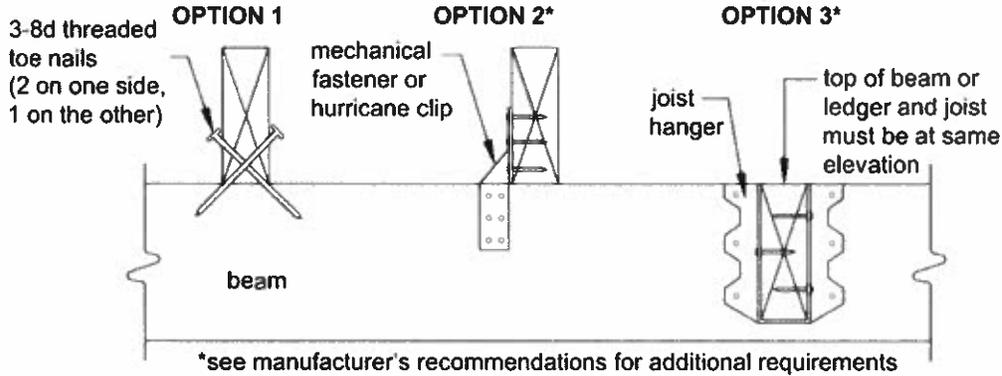


JOIST-TO-BEAM CONNECTION

Each joist shall be attached to the beam as shown in Figure 6. Joists may bear on and overhang past the beam face the lesser of L_0 or $L/4$ when Option 1 or Option 2 is used to attach the joist to the beam and blocking is provided between joists at beam bearing. Mechanical fasteners or hurricane clips used, as shown in Option 2,

must have a minimum capacity of 100 lbs in both uplift and lateral load directions. Joists may also attach to the side of the beam with joist hangers per Option 3. Joists shall not frame in from opposite sides of the same beam. See JOIST HANGERS for more information. Hangers, clips, and mechanical fasteners shall be galvanized or stainless steel (see MINIMUM REQUIREMENTS).

Figure 6. Joist-to-Beam Detail.



JOIST HANGERS

Joist hangers, as shown in Figure 7, shall have a depth of at least 60% of ledger or beam depth. Each hanger shall have a minimum vertical capacity in accordance with Table 3A. The joist hanger shall be selected from an *approved* manufacturer's product data based on the dimensions of the joist or header it is carrying. Joist hangers and fasteners shall be corrosion resistant (see MINIMUM REQUIREMENTS).

Use joist hangers with inside flanges when clearances to the edge of the beam or ledger board dictate. **Do not use clip angles or brackets to support joists.**

Figure 7. Typical Joist Hangers.

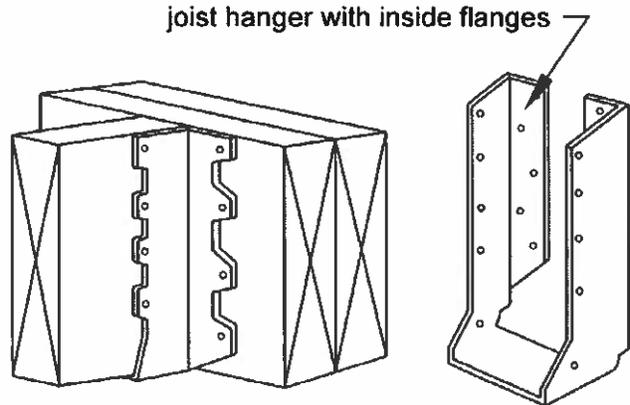


Table 3A. Joist Hanger Vertical Capacity.

Joist Size	Minimum Capacity, lbs
2x6	400
2x8	500
2x10	600
2x12	700

POST REQUIREMENTS

All deck post sizes shall be 6x6 (nominal) or larger, and the maximum height shall be in accordance with Table 4 and measured from grade or top of foundation, whichever is highest, to the underside of the beam. Under prescriptive limits of this document, 8x8 nominal posts can be substituted anywhere in Table 4 but are limited to a maximum height of 14'-0". Posts shall be centered on footings. Cut ends and notches of posts shall be field treated with an *approved* preservative (such as copper naphthenate) [R402.1.2]. The beam shall be attached to the post by notching as shown in Figure 8A or by providing an *approved* post cap to connect the beam and post as shown in Figure 8B. All 3-ply beams shall be connected to the post by a post cap. All through-bolts shall have washers under the bolt head and nut. Attachment of the beam to the side of the post without notching is prohibited (see Figure 9).

Provide diagonal bracing parallel to the beam at each corner post greater than 2'-0" in height as shown in Figure 10. Diagonal bracing is prohibited on center posts. Bracing shall be fastened to the post at one end and the beam at the other with 1/2" diameter lag screws. For non-ledger decks, (see Figure 21) diagonal bracing may be omitted at the beam and posts adjacent to the house.

Figure 8A. Post-to-Beam Attachment Requirements.

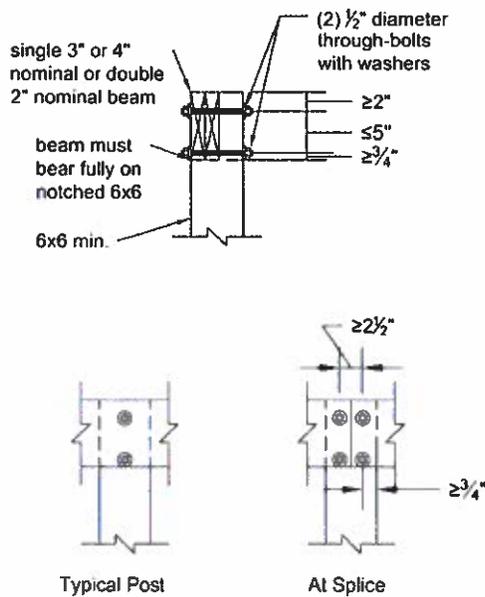


Figure 8B. Alternate Approved Post-to-Beam Post Cap Attachment.

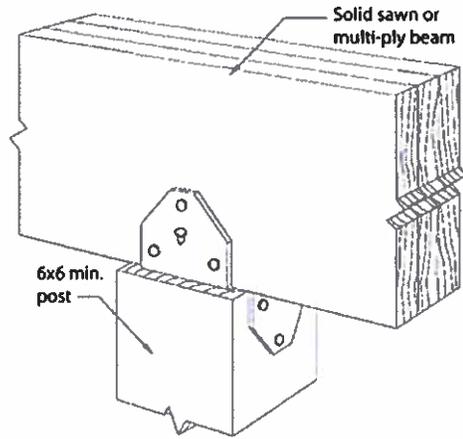


Figure 9. Prohibited Post-to-Beam Attachment Condition.

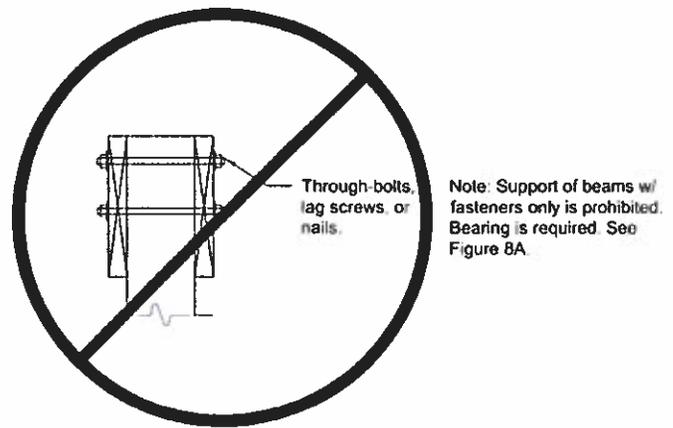
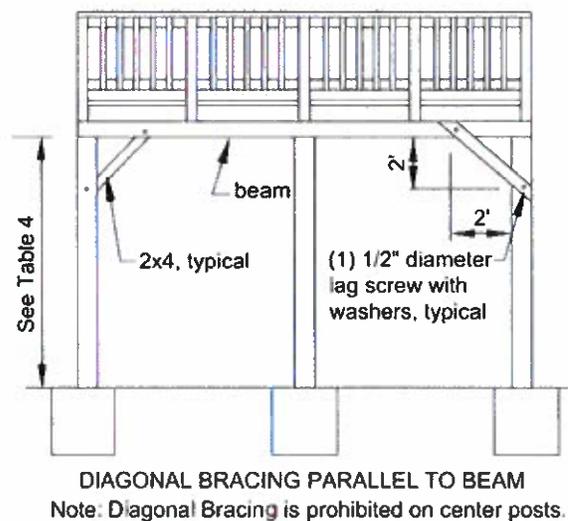


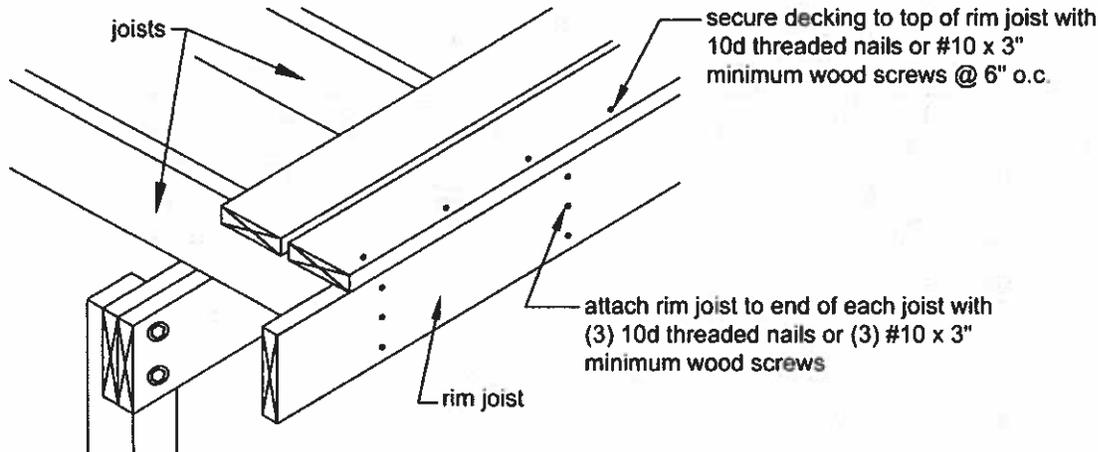
Figure 10. Diagonal Bracing.



RIM JOIST REQUIREMENTS

Attach a continuous rim joist to the ends of joists as shown in Figure 11. Attach decking to the rim joist as shown in Figure 11. For more decking attachment requirements, see DECKING REQUIREMENTS.

Figure 11. Rim Joist Connection Details.



FOOTINGS [R403]

See Figure 12 and Table 4 for footing size, footing thickness, and post attachment options and requirements. All footings shall bear on undisturbed soil at least 12 inches below the undisturbed ground surface or below the frost line, whichever is deeper. Contact the authority having jurisdiction to determine the specified frost line. Bearing conditions shall be verified in the field by the building official prior to placement of concrete. Where the building official determines that in-place soils with an allowable bearing capacity of less than 1,500 psf are likely to be present at the site, the allowable bearing

capacity shall be determined by a soils investigation. DECK FOOTINGS CLOSER THAN 5'-0" TO AN EXTERIOR HOUSE FOUNDATION WALL MUST BEAR AT THE SAME ELEVATION AS THE FOOTING OF THE HOUSE FOUNDATION.

Do not construct footings over septic systems or leach fields, utility lines, or enclosed meters. Contact local utilities (call 811) before digging.

Pre-manufactured post anchors shall be galvanized or stainless steel. See MINIMUM REQUIREMENTS.

Table 4. Post Height for 6x6⁵ and Footing Sizes for all Posts.

Beam Span, L _B	Joist Span L _J	Post Heights ¹					Footing Sizes ²		
		Southern Pine	Douglas Fir-Larch ³	Hem-Fir ³ , Western Cedars	Redwood	Ponderosa Pine, Red Pine, SPF ³	Round Footing Diameter	Square Footing	Footing Thickness ⁴
6'	≤10'	14'	14'	14'	14'	14'	18"	16"x16"	7"
	≤14'	14'	14'	14'	14'	14'	21"	18"x18"	8"
	≤18'	14'	14'	12'	14'	11'	24"	21"x21"	10"
8'	≤10'	14'	14'	14'	14'	14'	20"	18"x18"	8"
	≤14'	14'	14'	14'	14'	11'	24"	21"x21"	10"
	≤18'	14'	13'	11'	12'	8'	27"	24"x24"	11"
10'	≤10'	14'	14'	14'	14'	12'	23"	20"x20"	9"
	≤14'	14'	13'	11'	13'	8'	27"	24"x24"	11"
	≤18'	12'	11'	8'	11'	2'	31"	27"x27"	13"
12'	≤10'	14'	14'	12'	14'	10'	25"	22"x22"	10"
	≤14'	13'	12'	9'	11'	5'	30"	26"x26"	13"
	≤18'	11'	9'	6'	9'	2'	34"	30"x30"	15"
14'	≤10'	14'	13'	11'	13'	8'	27"	24"x24"	11"
	≤14'	11'	10'	7'	10'	2'	32"	29"x29"	14"
	≤18'	9'	8'	2'	8'	NP	37"	33"x33"	16"
16'	≤10'	13'	12'	10'	12'	6'	29"	26"x26"	12"
	≤14'	10'	9'	5'	9'	2'	35"	31"x31"	15"
	≤18'	7'	5'	2'	7'	NP	40"	35"x35"	18"
18'	≤10'	12'	11'	8'	11'	2'	31"	27"x27"	13"
	≤14'	9'	8'	2'	8'	NP	37"	33"x33"	16"
	≤18'	5'	2'	2'	6'	NP	42"	37"x37"	19"

1. Assumes 40 psf live load, 10 psf dead load, L_B/4 and L_J/4 overhangs, No 2. grade and wet service conditions.

2. Assumes 1,500 psf soil bearing capacity and 150 pcf concrete. Value may be multiplied by 0.9 for corner posts.

3. Incising assumed for Douglas Fir-Larch, Hem-Fir, and Spruce-Pine-Fir.

4. Assumes 2,500 psi compressive strength of concrete. Coordinate footing thickness with post base and anchor requirements.

5. 8x8 nominal posts may be substituted anywhere in Table 4 to a maximum height of 14'.

Figure 13A. Wood I-Joist Profile.

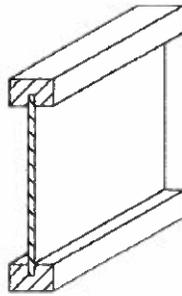


Figure 13B. Metal Plate Connected (MPC) Wood Floor Trusses with a 2x4 Lumber "Ribbon" at the Ends of the Trusses.

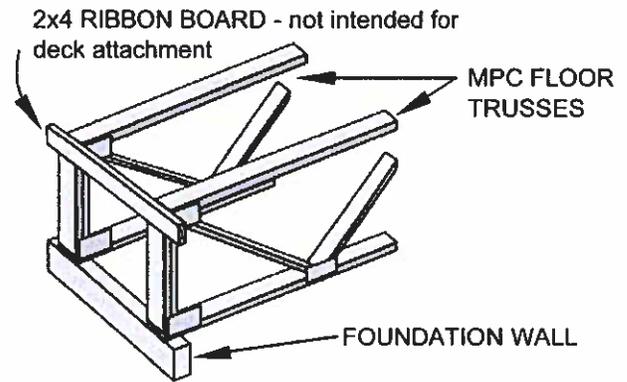


Figure 14. General Attachment of Ledger Board to Band Joist or Rim Joist.

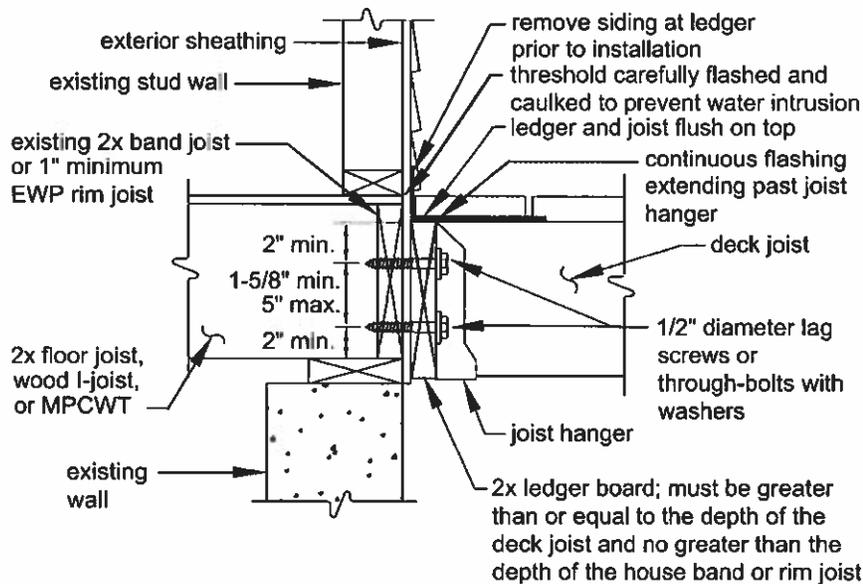
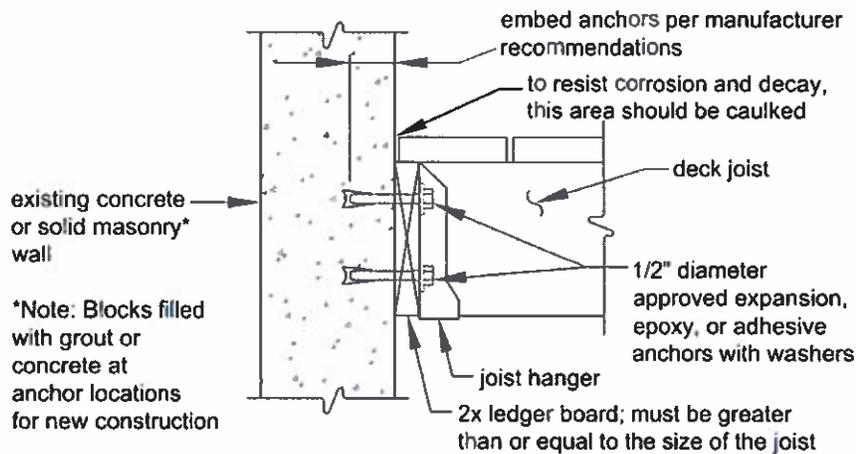


Figure 15. Attachment of Ledger Board to Foundation Wall (Concrete or Solid Masonry).



PROHIBITED LEDGER ATTACHMENTS

Attachments to exterior veneers (brick, masonry, stone), hollow masonry, and to cantilevered floor overhangs or bay windows are prohibited (see Figures 17 and 18). In such cases, the non-ledger deck is required (See NON-LEDGER DECKS).

Figure 17. No Attachment to or Through Exterior Veneers (Brick, Masonry, Stone).

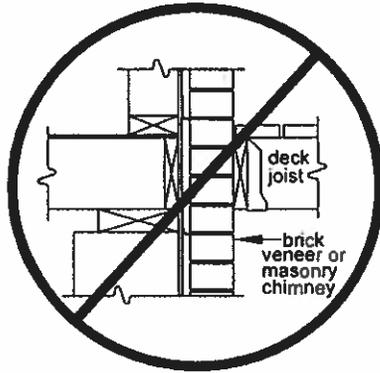


Figure 18. No Attachment to House Overhang with Ledger.



LEDGER BOARD FASTENERS

Only those fasteners noted below are permitted. **LEAD ANCHORS ARE PROHIBITED.**

Deck ledger connection to band joist or rim joist. The connection between a deck ledger and a 2-inch nominal

lumber band joist (1-1/2" actual) or LVL rim joist bearing on a sill plate or wall plate shall be constructed with 1/2" lag screws or bolts with washers per Table 5 and Figure 19 (see MINIMUM REQUIREMENTS).

Table 5. Fastener Spacing for a Southern Pine, Douglas Fir-Larch, or Hem-Fir Deck Ledger or Band or Rim Joist and a 2-inch Nominal Solid-Sawn Spruce-Pine-Fir Band Joist or LVL Rim Joist.^{3,4,5,6,8} (Deck Live Load = 40 psf, Deck Dead Load = 10 psf)

Connection Details	Rim Joist or Band Joist	Joist Span						
		6'-0" and less	6'-1" to 8'-0"	8'-1" to 10'-0"	10'-1" to 12'-0"	12'-1" to 14'-0"	14'-1" to 16'-0"	16'-1" to 18'-0"
On-Center Spacing of Fasteners								
1/2" diameter lag screw ¹ with 15/32" maximum sheathing	1" LVL	24"	18"	14"	12"	10"	9"	8"
	1-1/8" LVL	28"	21"	16"	14"	12"	10"	9"
	1-1/2" Lumber	30"	23"	18"	15"	13"	11"	10"
1/2" diameter bolt with 15/32" maximum sheathing	1" LVL	24"	18"	14"	12"	10"	9"	8"
	1-1/8" LVL	28"	21"	16"	14"	12"	10"	9"
	1-1/2" Lumber	36"	36"	34"	29"	24"	21"	19"
1/2" diameter bolt with 15/32" maximum sheathing and 1/2" stacked washers ^{2,7}	1-1/2" Lumber	36"	36"	29"	24"	21"	18"	16"

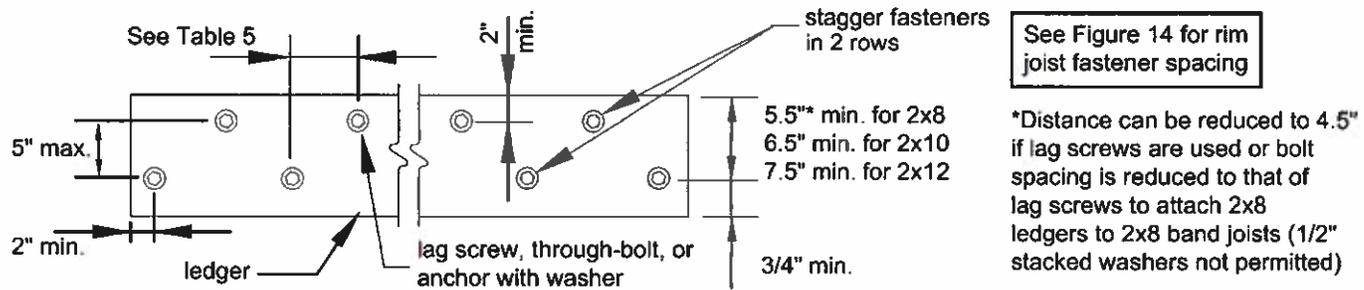
- The tip of the lag screw shall fully extend beyond the inside face of the band or rim joist.
- The maximum gap between the face of the ledger board and face of the wall sheathing shall be 1/2".
- Ledgers shall be flashed or caulked to prevent water from contacting the house band joist (see Figures 14 and 15).
- Lag screws and bolts shall be staggered per Figure 19.
- Deck ledgers shall be minimum 2x8 pressure-preservative-treated No.2 grade lumber, or other *approved* materials as established by standard engineering practice.
- When solid-sawn pressure-preservative-treated deck ledgers are attached to engineered wood products (minimum 1" thick wood structural panel band joist or structural composite lumber including laminated veneer lumber), the ledger attachment shall be designed in accordance with accepted engineering practice. Tabulated values based on 300 lbs and 350 lbs for 1" and 1-1/8" LVL rim joist, respectively.
- Wood structural panel sheathing, gypsum board sheathing, or foam sheathing shall be permitted between the band or rim joist and ledger. Stacked washers are permitted in combination with wood structural panel sheathing, but are not permitted in combination with gypsum board or foam sheathing. The maximum distance between the face of the ledger board and the face of the band joist shall be 1".
- Fastener spacing also applies to Southern Pine, Douglas Fir-Larch, and Hem-Fir band or rim joists.

Placement of lag screws or bolts in deck ledgers

The lag screws or bolts shall be placed as shown in Figure 19. The lag screws or bolts shall be staggered from the top to the bottom along the horizontal run of

the deck ledger (see Figure 19). Proper installation of lag screws or bolts shall be verified by the authority having jurisdiction.

Figure 19. Ledger Board Fastener Spacing and Clearances.



Through-Bolts

Through-bolts shall have a diameter of $\frac{1}{2}$ ". Pilot holes for through-bolts shall be $\frac{7}{32}$ " to $\frac{9}{16}$ " in diameter. Through-bolts require washers at the bolt head and nut.

Expansion and Adhesive Anchors

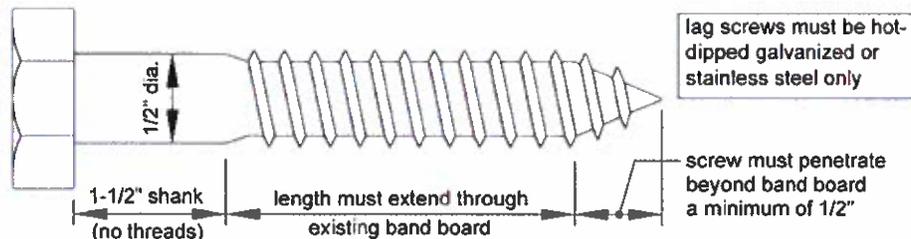
Use *approved* expansion or adhesive anchors when attaching a ledger board to a concrete or solid masonry wall as shown in Figure 15. Expansion and adhesive anchor bolts shall have a diameter of $\frac{1}{2}$ ". Minimum spacing and embedment length shall be per the

manufacturer's recommendations. All anchors must have washers.

Lag Screws

Lag screws shall have a diameter of $\frac{1}{2}$ " (see MINIMUM REQUIREMENTS). Lag screws may be used only when the field conditions conform to those shown in Figure 14. See Figure 20 for lag screw length and shank requirements. All lag screws shall be installed with washers.

Figure 20. Lag Screw Requirements.



Lag screw installation requirements: Each lag screw shall have pilot holes drilled as follows: 1) Drill a $\frac{1}{2}$ " diameter hole in the ledger board, 2) Drill a $\frac{5}{16}$ " diameter hole into the band board of the house. **DO NOT DRILL A $\frac{1}{2}$ " DIAMETER HOLE INTO THE BAND JOIST.**

The threaded portion of the lag screw shall be inserted into the pilot hole by turning. **DO NOT DRIVE LAG**

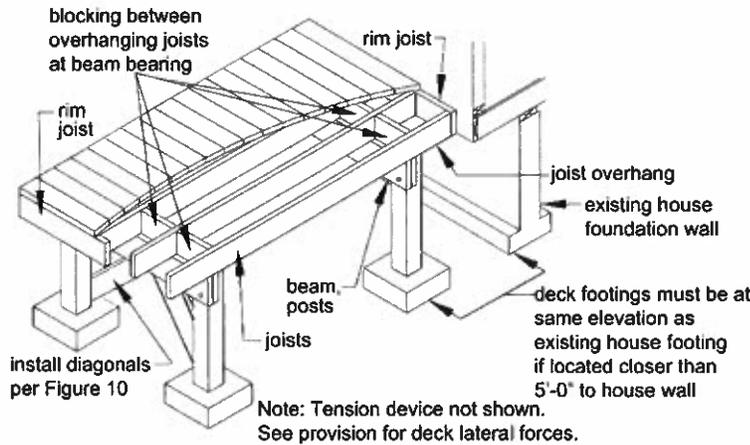
SCREWS WITH A HAMMER. Use soap or a wood-compatible lubricant as required to facilitate tightening. Each lag screw shall be thoroughly tightened (snug but not over-tightened to avoid wood damage).

NON-LEDGER DECKS - FOR RESISTING VERTICAL LOADS

Non-ledger decks use the house for resisting lateral loads but do not utilize the exterior wall of the house to support vertical loads (see Figure 21). Rather than supporting the deck on a ledger, an additional beam with posts is provided at or within the lesser of L_0 or $L/4$ of the house. THE ASSOCIATED DECK POST

FOOTINGS SHALL BE PLACED AT THE SAME ELEVATION AS THE HOUSE FOOTING IF LOCATED CLOSER THAN 5'-0" TO A HOUSE WALL (see Figure 2 and Figure 12). For houses with basements, a cylindrical footing (caisson) is recommended to minimize required excavation at the basement wall. Beam size is determined by Table 3. Non-ledger decks shall be attached to the house per Table 5 and Figures 22 or 23 for lateral loads.

Figure 21. Non-Ledger Deck - For Resisting Vertical Loads.



DECK LATERAL LOADS

Attachment to House: Decks shall be positively anchored to the primary structure [R507.1]. The lateral connection required shall be permitted to be in accordance with Figure 22 or 23 for ledger and non-ledger decks. Hold-down tension devices shall be provided in not less than two locations within two feet of the edge of the deck, and shall have an allowable stress design capacity of not less than 1,500 lb [R507.2.4].

For non-ledger decks, blocking or framing angles can be used in lieu of joist hangers and shall be provided on each side of each joist. Blocking shall be installed with 5-10d threaded nails into the rim joist or the framing angle shall have a lateral capacity of 600 lb. Flashing over the rim joist is required and must be installed in accordance with the flashing provisions in the LEDGER ATTACHMENT REQUIREMENTS.

The wall must be sheathed with minimum $\frac{3}{8}$ " wood structural panel sheathing. Use lag screws or through-bolts when fastening to a band joist; use expansion anchors or epoxy anchors when fastening to concrete or masonry. DO NOT ATTACH TO BRICK VENEERS. VERIFY THIS CONDITION IN THE FIELD PRIOR TO UTILIZING THIS METHOD. Fasteners shall penetrate beyond the house band board and be installed per Table 5.

Figure 22. Lateral Load Device with Floor Joists Parallel to Deck Joists.

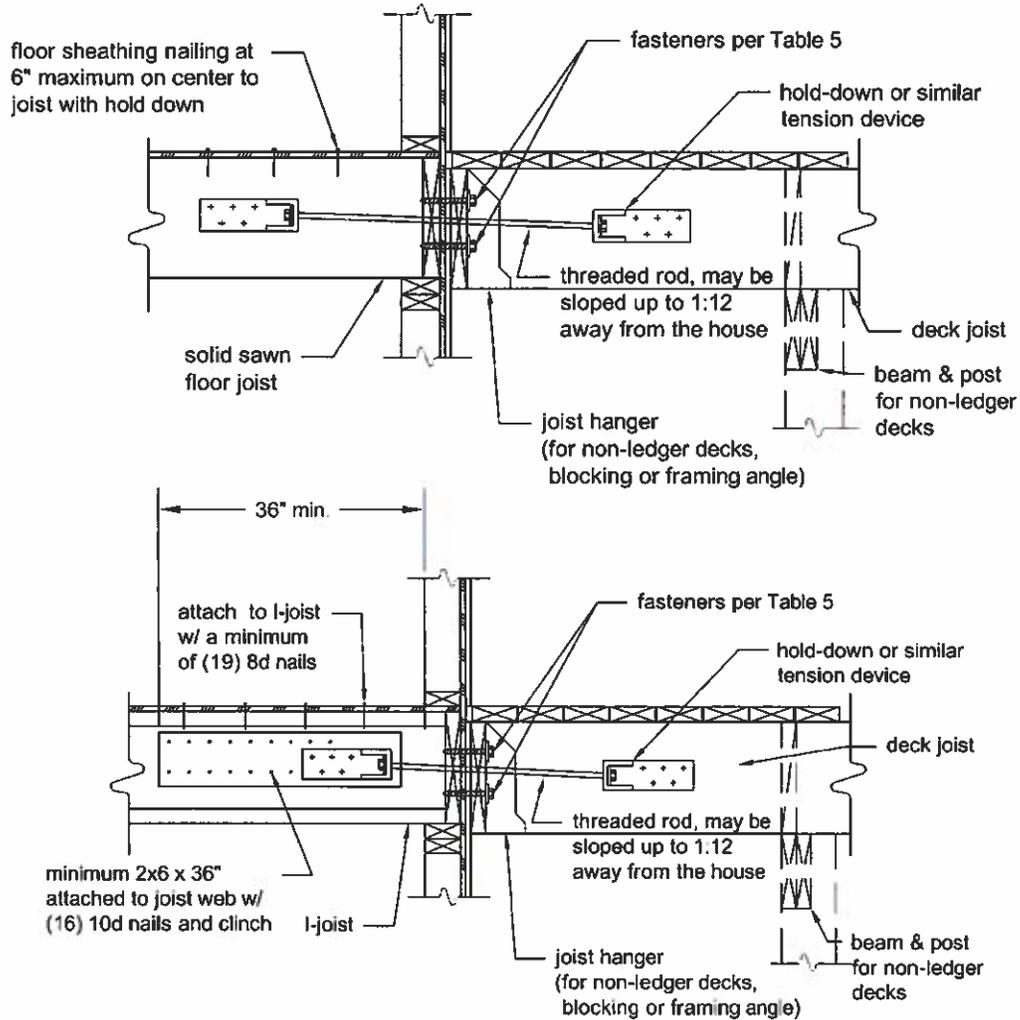
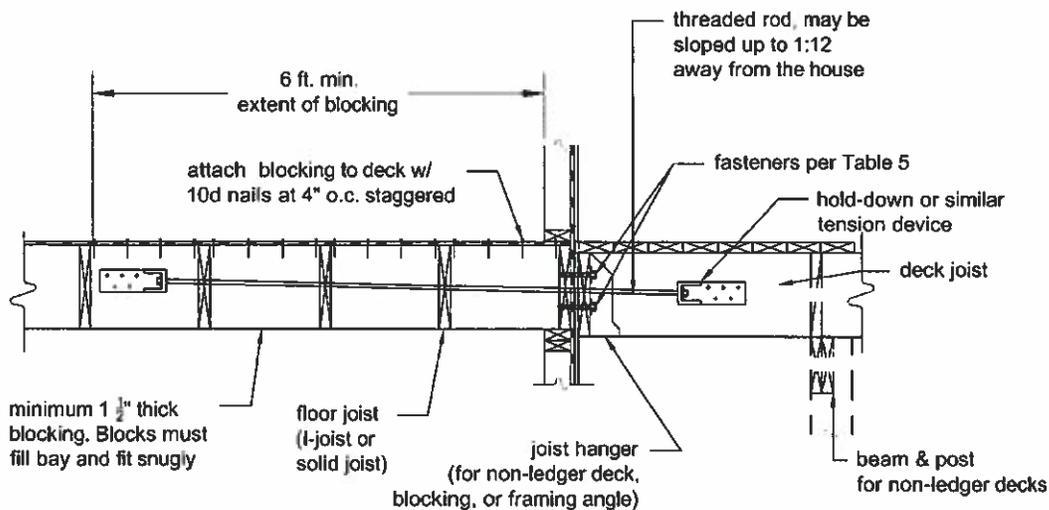


Figure 23. Lateral Load Device with Floor Joists Perpendicular to Deck Joists.

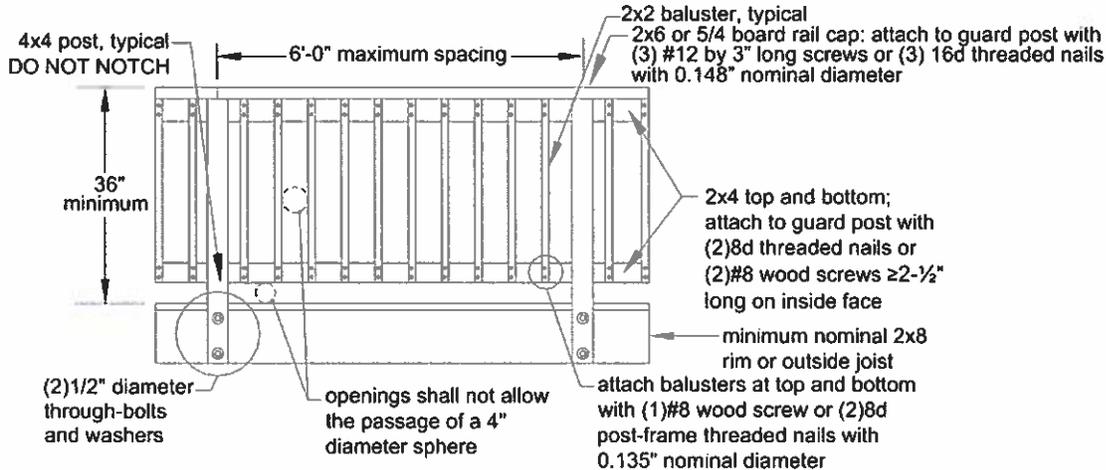


GUARD REQUIREMENTS

All decks greater than 30" above grade are required to have a guard [R312.1] - one example is shown in Figure 24. Other methods and materials may be used for guard

construction when *approved* by the authority having jurisdiction.

Figure 24. Example Guard Detail.



GUARD POST ATTACHMENTS FOR REQUIRED GUARDS

Deck guard posts for required guards shall be a minimum 4x4 (nominal) with an adjusted bending design value not less than 1,100 psi. Joists and rim joists to which guard posts are attached shall be a minimum of 2x8 (nominal).

Figure 25. Guard posts for required guards that run perpendicular to the deck joists shall be attached to the rim joist in accordance with Figure 26. Only hold-down anchor models meeting these minimum requirements shall be used. Hold-down anchors shall have a minimum allowable tension load of 1,800 pounds for a 36" maximum guard height and be installed in accordance with the manufacturer's instructions.

Guard posts for required guards which run parallel to the deck joists shall be attached to the outside joist per

Figure 25. Guard Post to Outside-Joist Example.

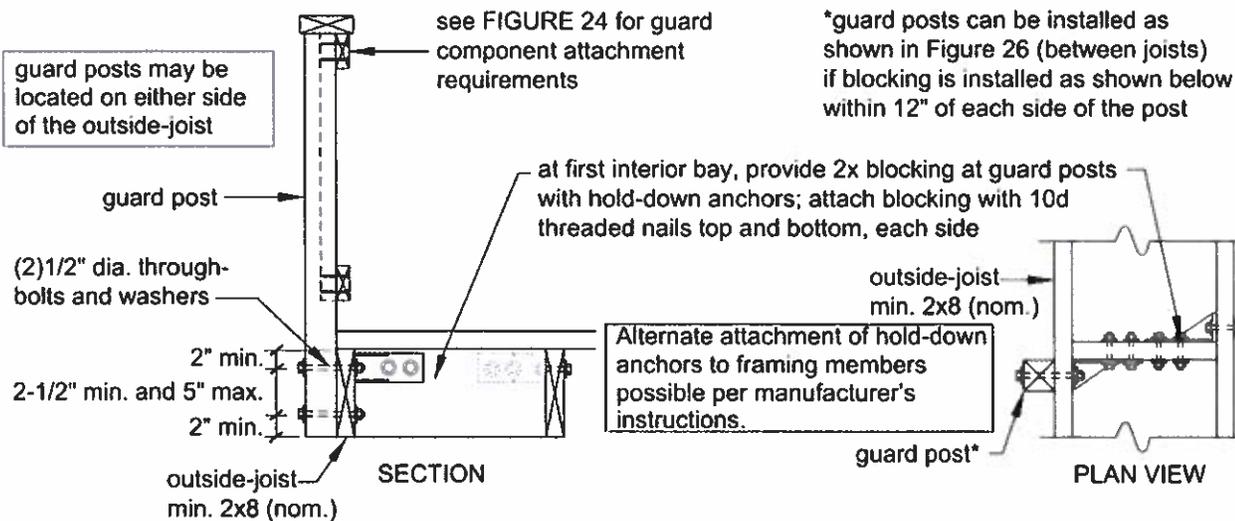
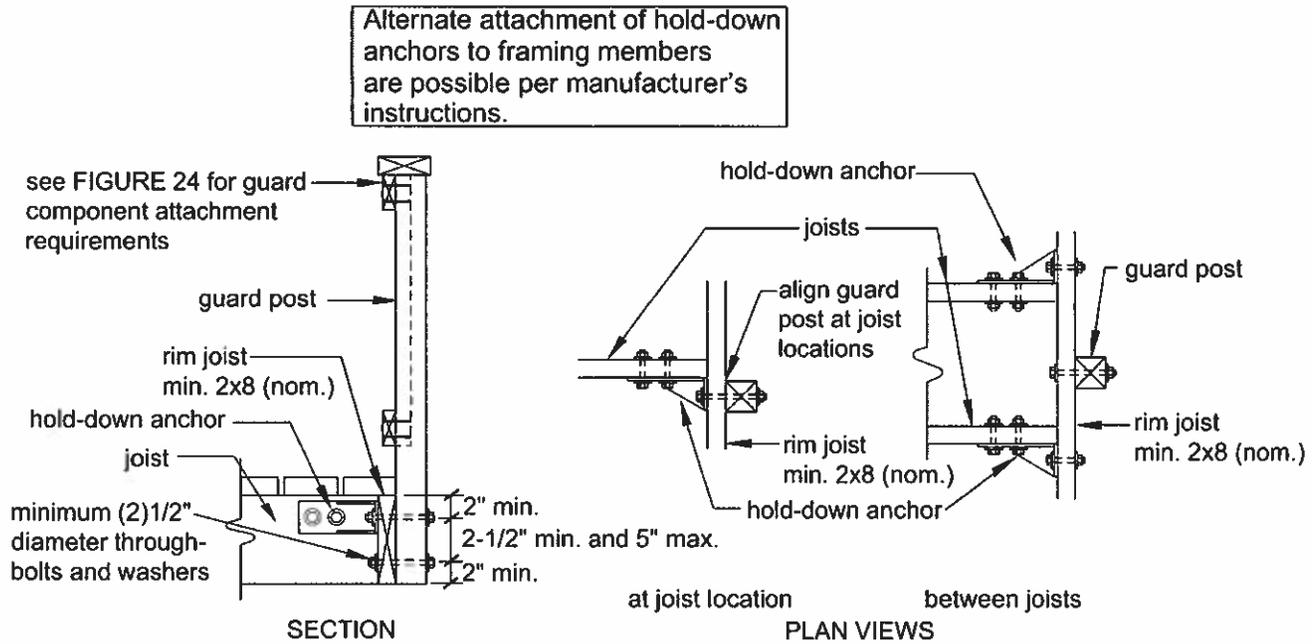


Figure 26. Guard Post to Rim Joist Example.



STAIR REQUIREMENTS

Stairs, stair stringers, and stair guards shall meet the requirements shown in Figure 27 through Figure 34 and Table 6 except where amended by the local jurisdiction. All stringers shall be a minimum of 2x12. Stair stringers shall not span more than the dimensions shown in Figure 28. If the stringer span exceeds these dimensions, then a 4x4 post may be provided to support the stringer and shorten its span length. The 4x4 post shall be notched and bolted to the stringer with (2) 1/2" diameter through-bolts with washers per Figure 8A. The post shall be centered on a 12" diameter or 10" square, 6" thick footing. The footing shall be constructed as shown in Figure 34 and attached to the post as shown in Figure 12. An intermediate landing may also be provided to shorten the stringer span (see provisions below). If the total vertical height of a stairway exceeds 12'-0", then an intermediate landing shall be required. All intermediate stair landings must be designed and constructed as a non-ledger deck using the details in this document. Stairs shall be a minimum of 36" in width as shown in Figure 33 [R311.7]. If only cut stringers are used, a minimum of three are required. For stairs greater than 36" in width, a combination of cut and solid stringers can be used, but shall be placed at a maximum spacing of 18" on center (see Figure 29). The width of each landing shall not be less than the width of the stairway served. Every rectangular landing shall have a minimum dimension of 36" measured in the direction of travel and no less than the width of the stairway served [R311.7].

Figure 27. Tread and Riser Detail.

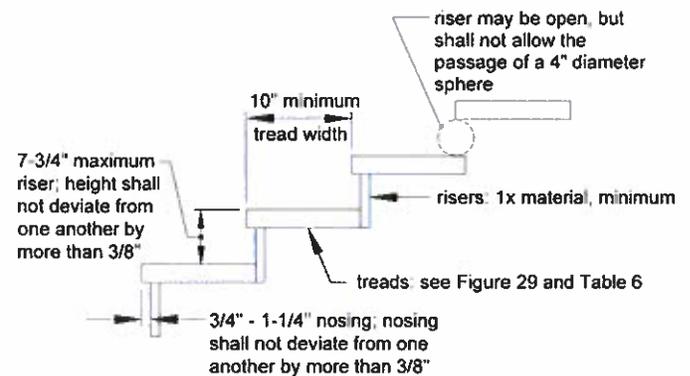


Figure 28. Stair Stringer Requirements.

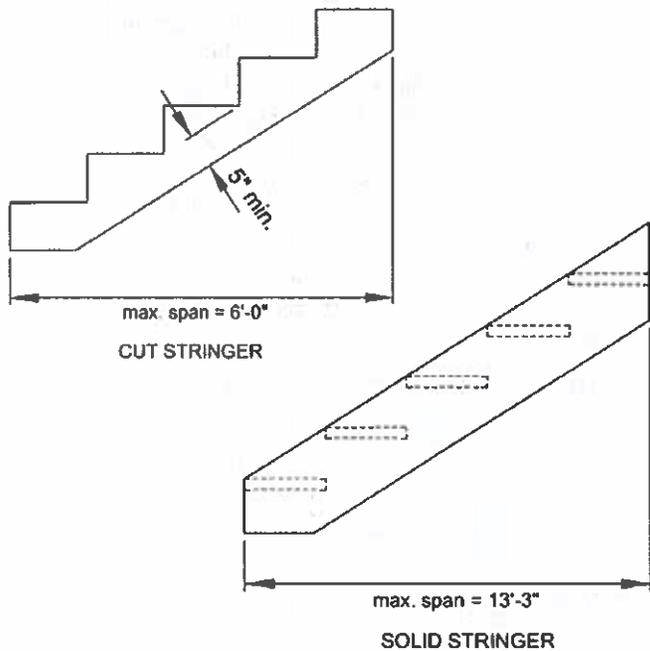


Figure 30. Stair Guard Requirements.

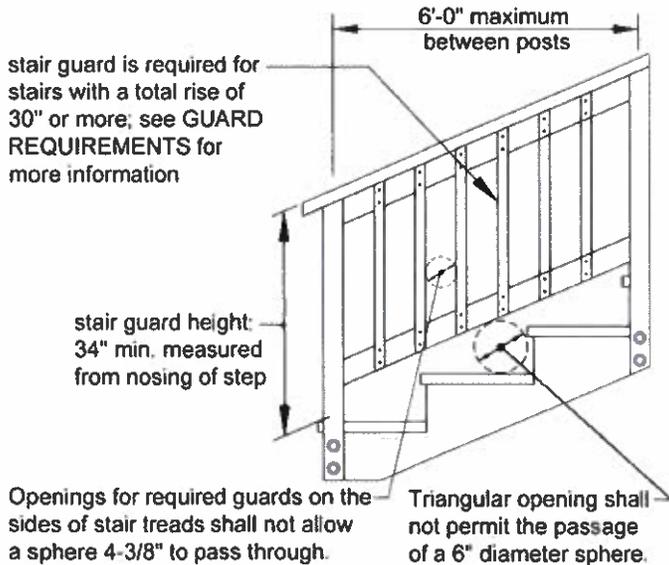


Figure 29. Tread Connection Requirements.

Attachment per tread at each stringer or ledger:
 2x_ or 5/4 treads - (2)#8d threaded nails or (2)#8 screws ≥2-1/2" long
 3x_ treads - (2)#16d threaded nails or (2)#8 screws ≥3-1/2" long

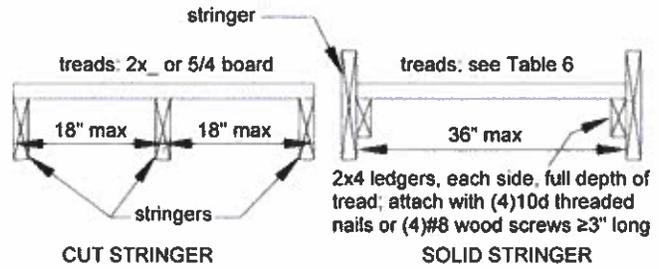


Figure 31. Stair Stringer Attachment Detail.

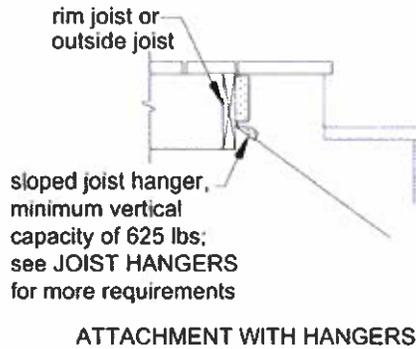


Table 6. Minimum Tread Size for Cut and Solid Stringers.¹

Species	Cut Stringer	Solid Stringer
Southern Pine	2x4 or 5/4	2x8
Douglas Fir Larch, Hem-Fir, SPF ²	2x4 or 5/4	2x8 or 3x4
Redwood, Western Cedars, Ponderosa Pine, ³ Red Pine ³	2x4 or 5/4	2x10 or 3x4

1. Assumes 300 lb concentrated load, L/288 deflection limit, No. 2 stress grade, and wet service conditions.

2. Incising assumed for Douglas Fir-Larch, Hem-Fir, and Spruce-Pine-Fir.

3. Design values based on northern species with no incising assumed.

STAIR HANDRAIL REQUIREMENTS

All stairs with 4 or more risers shall have a handrail on at least one side (see Figure 32A) [R311.7.8]. The handrail height measured vertically from the sloped plane adjoining the tread nosing shall be not less than 34 inches and not more than 38 inches (see Figure 30) [R311.7.8.1]. Handrails shall be graspable and shall be composed of decay-resistant and/or corrosion resistant material. Handrails shall be Type I, Type II, or provide equivalent graspability (see Figure 32B). Type I shall have a perimeter dimension of at least 4" and not greater than 6-1/4". Type II rails with a perimeter greater than 6-1/4" shall provide a graspable finger recess area on both sides of the profile [R311.7.8.3]. All shapes shall have a smooth surface with no sharp corners. Handrails shall run continuously from a point directly over the lowest riser to a point directly over the highest riser and shall return to the guard at each end (see Figure 33). Handrails may be interrupted by guard posts at a turn in the stair [R311.7.8.2].

Figure 32A. Handrail Mounting Examples.

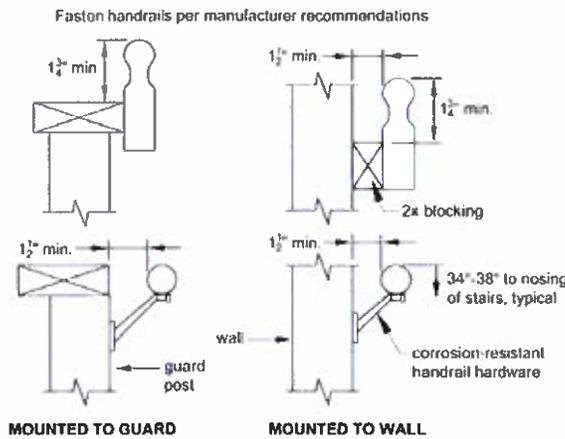
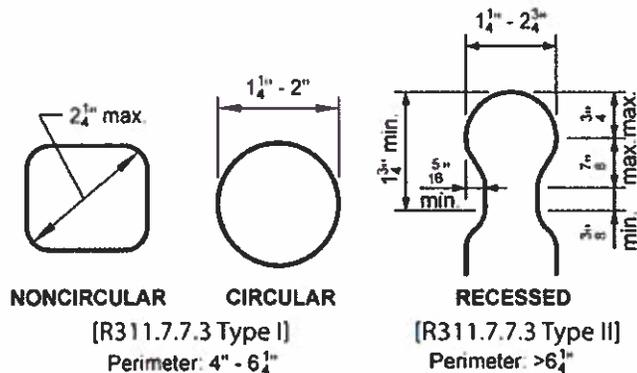


Figure 32B. Handrail Grip Size.



STAIR FOOTING REQUIREMENTS [R403]

Where the stairway meets grade, attach the stringers to the stair guard posts as shown in Figure 34. Posts shall bear on footings. All footings shall bear on solid ground and shall be placed at least 12 inches below the undisturbed ground surface or below the frost line, whichever is deeper (see Figure 34). Stringers shall bear on a 2x4 bearing block attached to the post as shown. Stringers shall not bear on new or existing concrete pads or patios that are not founded below this depth. When guards are not required (see GUARD REQUIREMENTS), posts may terminate below the bottom tread elevation. Bolts are only required if a guard post is required.

STAIR LIGHTING REQUIREMENTS [R303.7]

Stairways shall have a light source located at the top landing such that all stairs and landings are illuminated. The light switch shall be operated from inside the house. However, motion detected or timed switches are acceptable.

Figure 33. Miscellaneous Stair Requirements.

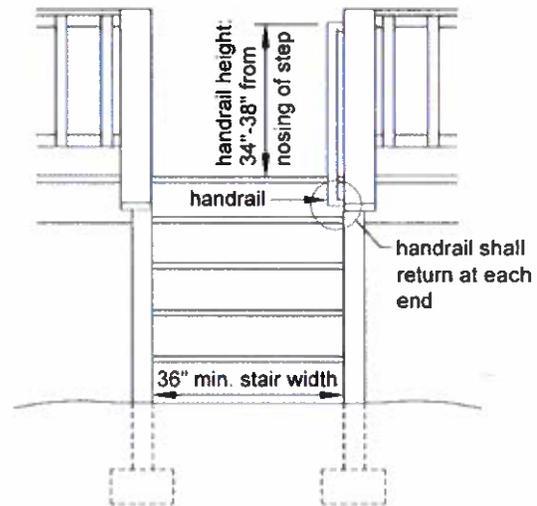
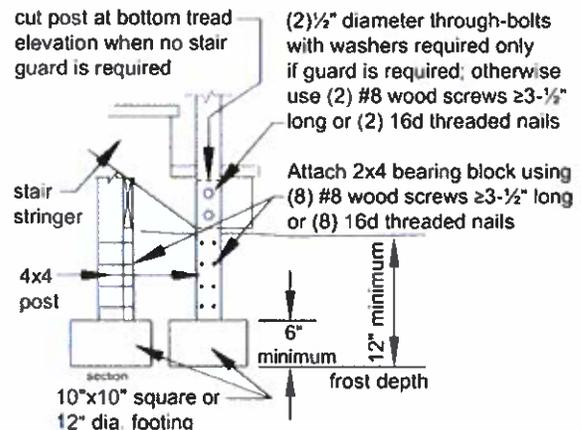


Figure 34. Stair Footing Detail.



FRAMING AT CHIMNEY OR BAY WINDOW

All members at a chimney or bay window shall be framed in accordance with Figure 35. Headers may span a maximum of 6'-0". When a chimney or bay window is wider than 6'-0", one or more 6x6 posts may be added to reduce header spans to less than 6'-0". In such cases, the post footing must meet the requirements in the FOOTINGS section. Headers shall be located no more than 3'-0" from the end of the trimmer joist.

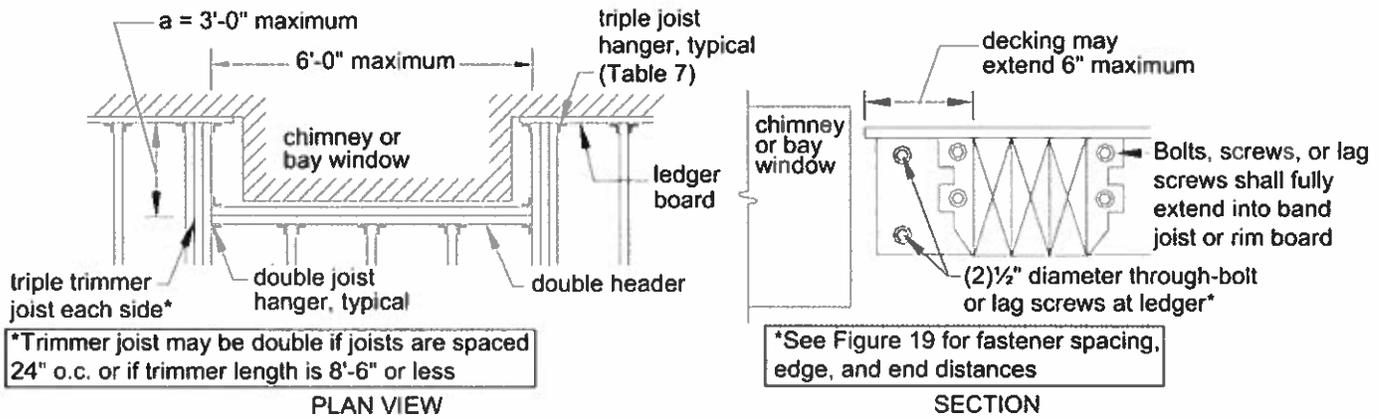
Triple trimmer joists are required on each side of the header if joist spacing is 12" or 16" o.c. or if the trimmer joist span exceeds 8'-6"; otherwise, double trimmer joists are permitted. Trimmer joists may bear on the beam and extend past the beam centerline up to the lesser of L_0 or $L/4$ as shown in Figures 1A and 2, or the trimmer joist may attach to the side of the beam with joist hangers as

shown in Figure 1B. Joist hangers shall each have a minimum vertical capacity in accordance with Table 7. Bolts, screws, or lag screws used to attach the hanger to the ledger shall fully extend through the ledger into the 2-inch nominal lumber band joist (1-1/2" actual) or LVL rim joist. Otherwise a non-ledger deck is required.

Table 7. Trimmer Joist Hanger Vertical Capacity.

Joist Size	Minimum Capacity, lbs
2x6	870
2x8	1155
2x10	1420
2x12	1575

Figure 35. Detail for Framing Around a Chimney or Bay Window.



REFERENCES

2015 *International Residential Code (IRC)*, International Code Council (ICC), Washington, DC, 2015.

ANSI/ASME Standard B18.2.1-2010, Square, Hex, Heavy Hex, and Askew Head Bolts and Hex, Heavy Hex, Hex Flange, Lobed Head, and Lag Screws (Inch Series), American Society of Mechanical Engineers, New York, NY, 2010.

ANSI/ASME Standard B18.6.1-1981 (Reaffirmed 1997), Wood Screws (Inch Series), American Society of Mechanical Engineers, New York, NY, 1982.

ASTM A 123-17, Specification for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Products, ASTM, West Conshohocken, PA 2017.

ASTM Standard A 153-16a, Standard Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware, ASTM, West Conshohocken, PA 2016.

ASTM A 653-17, Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process, ASTM, West Conshohocken, PA 2017.

ASTM Standard B 695-04, Standard Specification for Coatings of Zinc Mechanically Deposited on Iron and Steel, ASTM, West Conshohocken, PA 2016.

ASTM F 1667-17, Standard Specification for Driven Fasteners: Nails, Spikes, and Staples, ASTM, West Conshohocken, PA 2017.

Attachment of Residential Deck Ledger to Metal Plate Connected Wood Truss Floor System, Structural Building Components Association (SBCA), www.sbcindustry.com, 2015.

AWPA Book of Standards, American Wood Protection Association, Birmingham, AL, 2017.

Policy for Evaluation of Recommended Spans for Span Rated Decking Products, American Lumber Standard Committee, Germantown, MD 2004.



AMERICAN
WOOD
COUNCIL



It is intended that this document be used in conjunction with competent design, accurate fabrication, and adequate supervision of construction. While every effort has been made to insure the accuracy of the information presented, the American Wood Council and its members do not assume responsibility for errors or omissions in the document, nor for designs, plans, or construction prepared from it. Those using this document assume all liability arising from its use.

Copyright © 2007, 2009, 2010, 2014, 2015, 2018
American Wood Council

04-18

Design for
Code Acceptance



**Appendix A to
Prescriptive Residential
Wood Deck Construction Guide
DCA 6**

SAFETY GLAZING REQUIREMENTS: *IRC* R308.4 states that safety glazing in window glass is required when the existing house wall acts as a barrier to adjacent stairs, landings, and areas at the top and bottom of stairs. If a window or portion thereof falls within the area shown in Figure A1, glass panes within that area should be safety glazed. Safety glazing should reduce injury due to accidental impact when ascending or descending stairs. Application of safety glazing film to glass that was not originally treated is acceptable to meet this requirement.

Exceptions to this requirement include:

- When a protective bar is installed on the accessible side(s) of the glazing 36" ± 2" above the deck surface. The bar shall be capable of withstanding a horizontal load of 50 plf without contacting the glass and be a minimum of 1-½" in height.
- The side of a stairway, landing, or ramp has a guardrail or handrail, including balusters or in-fill panels, complying with the provisions of Sections R311.7.8 and R312 and the plane of the glass is more than 18" from the railing.
- When a solid wall or panel extends from the plane of the adjacent walking surface to 34" - 36" above the floor and the construction at the top of that wall or panel is capable of withstanding the same horizontal load as the protective bar.

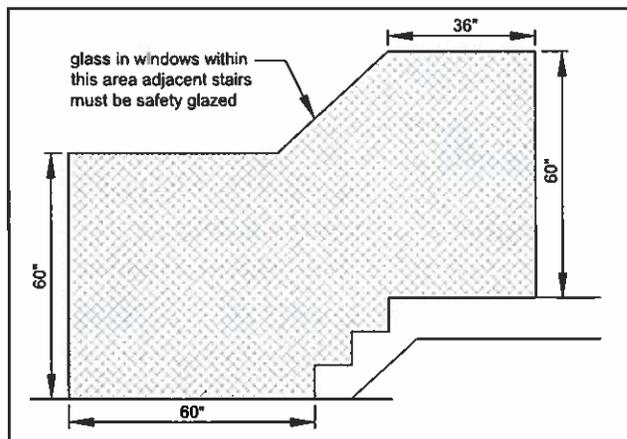


Figure A1. Safety glazing requirements.



**Appendix B to
Prescriptive Residential
Wood Deck Construction Guide
DCA 6**

This Appendix to DCA 6 – Prescriptive Residential Wood Deck Construction Guide has been requested by builders, building officials, and others, to provide an alternative to the assumptions of Table 4 Post Height for 6x6 and Footings Sizes for all Posts. DCA 6 is written to be as simple and easy to use as possible; however, this means that conservative assumptions often have to be made. This is especially true for Table 4, which assumes full cantilevers on both the joist and the beam. If full cantilevers are not present, the load will be less than assumed in Table 4, and the footing size will often decrease and allowable post height will often increase. This Appendix is an aid to calculating the loads on posts and footings based on the actual deck configuration.

Tributary Area

Since uniform loading is prescriptively set for DCA 6, only the area the post or footing is supporting needs to be determined. This is called the tributary area and is shown in Figure B1. Tributary area of a center or corner post can be found from Figure B1 or by using the following formulas:

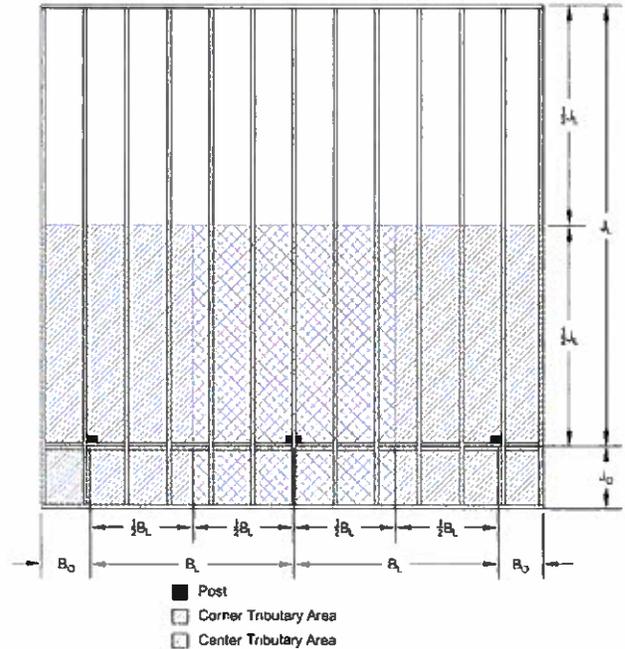
$$A_{\text{CenterPost}} = \left(\frac{1}{2}J_L + J_O\right)(B_L) \quad \text{Eq. B-1}$$

$$A_{\text{CornerPost}} = \left(\frac{1}{2}J_L + J_O\right)\left(\frac{1}{2}B_L + B_O\right) \quad \text{Eq. B-2}$$

Where:

- A is tributary area (ft²)
- J_L is length of joist (ft)
- J_O is length of joist overhang (ft)
- B_L is the length of the beam span (ft)
- B_O is the length of the beam overhang (ft)

Figure B1. Examples of Tributary Areas



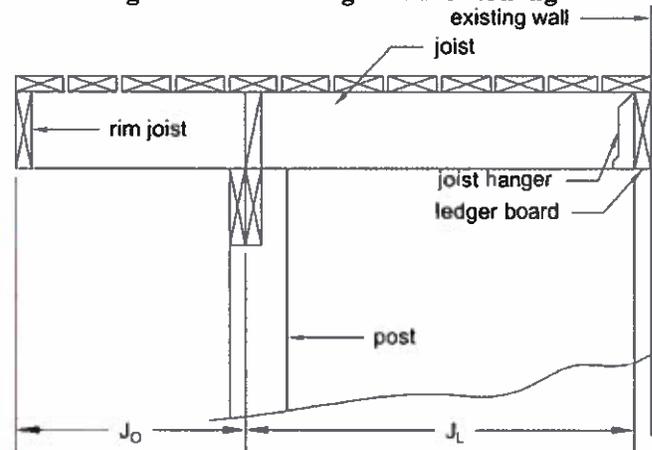
Joist Length, J_L

The joist length is defined differently for this appendix than the main provisions of DCA 6. The joist length is not the design span of the joist, but is from the ledger face to either the center point of the beam, if there is an overhang, or to the outside face of the rimboard if there is not an overhang. See Figure B2.

Joist Overhang Length, J_O

The length of the joist overhang is measure from the outside edge of the deck to the centerline of the beam. See Figure B2. If no overhang exists, zero is entered into equation B-1 or B-2 for J_O.

Figure B2. Joist Length and Overhang



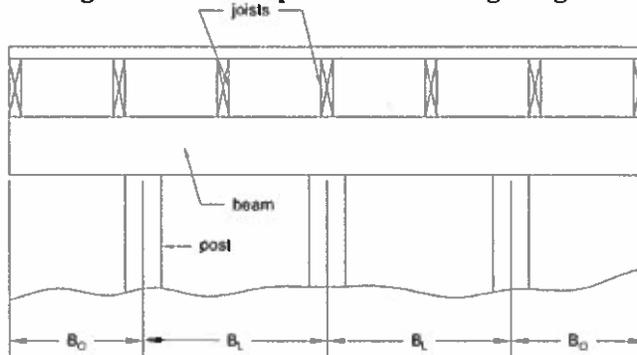
Beam Span Length, B_L

The beam span is measured from either centerline of post to centerline of post, if there are overhangs, or to the outside edges of the deck, if there are no overhangs. For posts or footings being considered with two unequal, adjacent beam spans, the greater span shall be used. See Figure B3.

Beam Overhang Length, B_O

The length of the beam overhang is measured from the outside edge of the deck to the centerline of the nearest post. See Figure B3. If a center post or footing is being considered or no overhang exists, zero is entered into the equation B-1 or B-2 for B_O .

Figure B3. Beam Span and Overhang Length



Post and Footings Size

As an alternative to Table 4 of *DCA 6*, the post height and footing size may be in accordance with Table B1 through B3.

Table B1. Post Heights Based^{1,3} on Tributary Area for Corner Posts.

Tributary Area (sq. ft.)	6x6 Post Height (ft.)					4x4 Post Height (ft.)				
	Southern Pine	Douglas Fir- Larch ²	Hem-Fir ² , Western Cedars	Redwood	Ponderosa Pine, Red Pine, SPF ²	Southern Pine	Douglas Fir- Larch ²	Hem-Fir ² , Western Cedars	Redwood	Ponderosa Pine, Red Pine, SPF ²
10	14	14	14	14	14	9	7	7	11	8
20	14	14	14	14	14	6	4	5	7	5
30	14	14	14	14	14	5	3	4	6	4
40	14	14	14	14	14	4	3	3	5	3
50	14	14	14	14	14	4	2	3	4	3
60	14	14	14	14	14	3	2	2	4	2
70	14	14	14	14	13	3	2	2	3	2
80	14	14	13	14	12	2	2	2	3	2
90	14	14	12	14	10	2	2	2	2	2
100	14	14	11	13	9	2	2	2	2	2
110	14	13	10	12	7	2	2	2	2	2
120	13	12	10	12	6	2	2	2	2	2
130	13	11	9	11	4	2	2	2	2	2
140	12	11	8	10	2	2	2	2	2	2
150	11	10	7	10	2	2	2	2	2	2
160	11	9	6	9	2	2	2	2	2	2
170	10	9	5	9	NP	2	2	2	2	2
180	10	8	3	9	NP	2	2	2	2	2
190	10	7	2	8	NP	2	2	2	2	2
200	8	7	2	8	NP	2	2	2	2	2
210	8	6	2	7	NP	2	2	2	2	2
220	7	5	2	7	NP	2	2	2	2	2
230	6	4	2	7	NP	2	2	2	2	2
240	6	2	2	6	NP	2	2	NP	2	2
250	5	2	2	6	NP	2	2	NP	NP	NP

1. Assumes No 2. Stress grade and wet service conditions.

2. Incising assumed for Douglas fir-larch, hem-fir, and spruce-pine-fir.

3. Some post heights for 4x4 post sizes show a greater load carrying capacity than 6x6 post sizes since different ASTM Standards are used to develop design values for visually graded dimension lumber vs. visually graded timbers.

Table B2. Post Heights Based^{1,3} on Tributary Area for Center Posts.

Tributary Area ⁴ (sq. ft.)	6x6 Post Height (ft.)					4x4 Post Height (ft.)				
	Southern Pine	Douglas Fir-Larch ²	Hem-Fir, Western Cedars	Redwood	Ponderosa Pine, Red Pine, SPF ²	Southern Pine	Douglas Fir-Larch ²	Hem-Fir, Western Cedars	Redwood	Ponderosa Pine, Red Pine, SPF ²
10	14	14	14	14	14	14	14	14	14	14
20	14	14	14	14	14	14	14	14	14	14
30	14	14	14	14	14	14	14	13	13	14
40	14	14	14	14	14	13	14	11	11	12
50	14	14	14	14	14	11	12	10	10	10
60	14	14	14	14	14	10	11	10	9	10
70	14	14	14	14	14	9	10	9	8	9
80	14	14	14	14	14	9	9	8	7	8
90	14	14	14	14	14	8	9	7	7	7
100	14	14	14	14	14	8	8	7	7	7
110	14	14	14	14	14	7	8	6	6	6
120	14	14	14	14	14	7	7	6	6	6
130	14	14	14	14	13	6	7	5	6	6
140	14	14	13	14	11	6	7	5	5	5
150	14	14	13	14	10	6	6	5	5	5
160	14	14	12	13	8	6	6	5	5	5
170	14	14	11	13	6	5	6	4	4	5
180	14	14	11	12	NP	5	6	4	4	4
190	13	13	11	12	NP	5	5	4	4	4
200	13	13	10	11	NP	4	5	3	4	4
210	12	12	8	11	NP	4	5	3	3	3
220	12	11	8	10	NP	4	5	2	3	3
230	11	11	7	10	NP	4	4	2	3	2
240	10	10	6	10	NP	3	4	NP	2	2
250	10	10	5	9	NP	3	4	NP	1	NP

1. Assumes No 2. Stress grade and wet service conditions.
2. Incising assumed for Douglas fir-larch, hem-fir, and spruce-pine-fir.
3. Some post heights for 4x4 post sizes show a greater load carrying capacity than 6x6 post sizes since different ASTM Standards are used to develop design values for visually graded dimension lumber vs. visually graded timbers.
4. Tributary area shall be multiplied by 1.25 at center posts with beams not spliced (continuous).

Table B3. Footing Sizes¹ Based on Tributary Area for Various Soil Capacities.

Tributary Area ² (sq. ft.)	Soil Bearing Capacity											
	1500 psf			2000 psf			2500 psf			3000 psf		
	Round Footing Diameter (in.)	Square Footing (in.)	Footing Thickness (in.)	Round Footing Diameter (in.)	Square Footing (in.)	Footing Thickness (in.)	Round Footing Diameter (in.)	Square Footing (in.)	Footing Thickness (in.)	Round Footing Diameter (in.)	Square Footing (in.)	Footing Thickness (in.)
10	8	7	6	7	7	6	7	6	6	6	5	6
20	12	10	6	10	9	6	9	8	6	8	7	6
30	14	13	6	12	11	6	11	10	6	10	9	6
40	16	15	6	14	13	6	13	11	6	12	10	6
50	18	16	7	16	14	6	14	13	6	13	12	6
60	20	18	8	17	15	6	16	14	6	14	13	6
70	22	19	9	19	17	7	17	15	6	15	14	6
80	23	21	9	20	18	8	18	16	7	16	15	6
90	25	22	10	21	19	8	19	17	7	17	15	6
100	26	23	11	23	20	9	20	18	8	18	16	7
110	28	25	12	24	21	10	21	19	8	19	17	7
120	29	26	12	25	22	10	22	19	9	20	18	8
130	30	27	13	26	23	11	23	20	9	21	18	8
140	31	28	13	27	24	11	24	21	10	22	19	9
150	33	29	14	28	25	12	25	22	10	22	20	9
160	34	30	15	29	25	12	25	23	10	23	20	9
170	35	31	15	30	26	13	26	23	11	24	21	10
180	36	32	16	30	27	13	27	24	11	24	22	10
190	37	33	16	31	28	13	28	25	12	25	22	10
200	38	34	17	32	29	14	28	25	12	26	23	11
210	39	35	17	33	29	14	29	26	12	26	23	11
220	40	35	18	34	30	15	30	26	13	27	24	11
230	41	36	18	35	31	15	31	27	13	28	25	12
240	42	37	19	35	31	15	31	28	13	28	25	12
250	43	38	19	36	32	16	32	28	14	29	26	12

1. Assumes 40 psf live load, 10 psf dead load, 150 pcf concrete and 2,500 psi compressive strength of concrete. Coordinate footing thickness with post base and anchor requirements.
2. Tributary area shall be multiplied by 1.25 at center posts with beams not spliced (continuous).